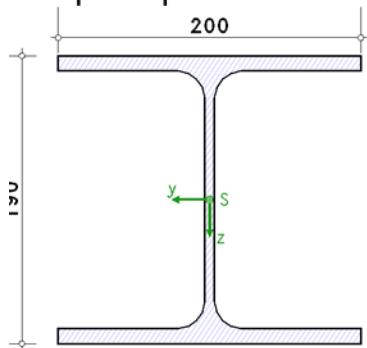


POS. 10: FIRE DESIGN EX. 5.1

fire design EC 3-1-2 (12.10), NA: Deutschland

1. input report



steel

steel grade S275

material safety factor

resistance of cross-sections $\gamma_{M0} = 1.00$

resistance of components in the event of fire $\gamma_{M,fi} = 1.00$

geometry

section HE200A

cross-section temperature

thermal action due to the standard curve, fire resistance time $t = 30 \text{ min}$

section all sides flamed

resistance

plastic verification

fire design at temperature level

adjustment factors for uneven temperature distribution

across the cross section $\kappa_1 = 1.00$, along the beam $\kappa_2 = 1.00$

internal forces and moments (event of fire)

σ -generating forces (N , M) work at centroid, τ -generating forces (V , T_t) work at shear center

Lk 1: $N_{fi} = 780.00 \text{ kN}$

notes

stability is not investigated.

2. cross-section temperature

surface of the section exposed to fire $A_m = 1136.1 \text{ mm}^2/\text{mm}$

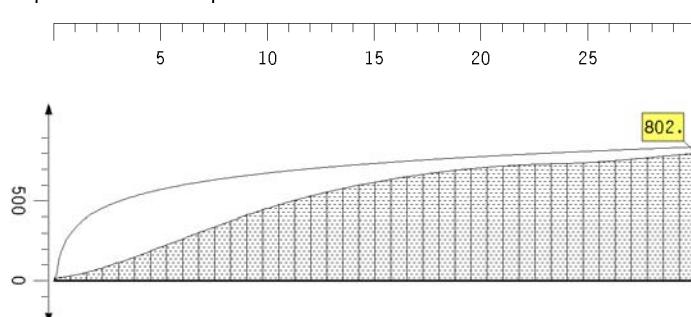
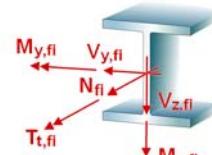
section factor of the unprotected component $A_m/V = 1136.1 / 5383.1 \cdot 10^3 = 211.0 \text{ 1/m}$

fire-stressed inner surface of the enclosing box $A_b = 780.0 \text{ mm}^2/\text{mm}$

section factor for the enclosing box $A_b/V = 780.0 / 5383.1 \cdot 10^3 = 144.9 \text{ 1/m}$

correction factor $k_{sh} = (A_b/V) / (A_m/V) = 144.9 / 211.0 = 0.687$, I-section: $0.9 \cdot k_{sh} = 0.618$

temperature development:



temperature in $^{\circ}\text{C}$
fire time in min
max $T_a = 801.9^{\circ}\text{C}$
max $t = 30 \text{ min}$

cross-section temperature acc. to $t = 30 \text{ min}$: $T_a = 801.9^{\circ}\text{C}$

3. Lk 1

3.1. fire design

internal forces and moments (event of fire): $N_{fi} = 780.00 \text{ kN}$

3.1.1. plastic verification

3.1.1.1. verification at temperature level

Utilisation rate at time $t = 0$ (normal temperature)

plastic verification for $N = 780.00 \text{ kN}$

max. load factor of normal stresses (plastic): $f_{\sigma,pl} = 1.896 \Rightarrow U_{\sigma,pl} = 0.528$

verification: $U_{pl} = 0.528 < 1 \text{ ok}$

critical temperature $T_{a,cr} = 39.19 \cdot \ln[1/(0.9674 \cdot \mu_0^{3.833}) - 1] + 482 = 576.0 \text{ }^{\circ}\text{C}$ with $\mu_0 = 0.528$

existing temperature $T_a = 801.9 \text{ }^{\circ}\text{C}$

verification: $U_T = T_a/T_{a,cr} = 1.392 > 1 \text{ fault !!}$

4. final result

maximum utilization:	temperature	$\max U_T = 1.392 > 1 \text{ fault !!}$
	stress	$\max U_{\sigma,t=0} = 0.528 < 1 \text{ ok}$
	resistance	$\max U = 1.392 > 1 \text{ fault !!}$

resistance not ensured !!

5. Regulations

DIN EN 1990, Eurocode 0: Grundlagen der Tragwerksplanung;

Deutsche Fassung EN 1990:2002 + A1:2005 + A1:2005/AC:2010, Ausgabe Dezember 2010

DIN EN 1990/NA, Nationaler Anhang zur DIN EN 1990, Ausgabe Dezember 2010

DIN EN 1991-1-2, Eurocode 1: Einwirkungen auf Tragwerke - Teil 1-2: Allgemeine Einwirkungen -

Brandeinwirkungen auf Tragwerke; Deutsche Fassung EN 1991-1-2, Ausgabe Dezember 2010

DIN EN 1991-1-2/NA, Nationaler Anhang zur DIN EN 1991-1-2, Ausgabe September 2015

DIN EN 1993-1-2, Eurocode 3: Bemessung und Konstruktion von Stahlbauten - Teil 1-2: Allgemeine Regeln -

Tragwerksbemessung für den Brandfall; Deutsche Fassung EN 1993-1-2, Ausgabe Dezember 2010

DIN EN 1993-1-2/NA, Nationaler Anhang zur DIN EN 1993-1-2, Ausgabe Dezember 2010