

## POS. 19: WAGENKNECHT BD.2, BSP.11.10.7

### detailed problems acc. to Eurocode 3

EC 3-1-5 (12.10), NA: Deutschland

#### steel grade

steel grade S 235

#### cross-section

buckling field: thickness  $t = 12.0$  mm, width  $b = 700.0$  mm

transverse stiffeners to limit the buckling field:

section L100X65X9

#### parameters

length of buckling field  $a = 200.0$  cm

method of reduced stresses

verification in beam field

calculation of buckling factors acc. to EC 3-1-5

#### loading

Lk 1:  $M_{Ed} = 69.6$  kNm  $V_{Ed} = 244.0$  kN

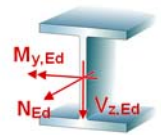
transverse loading on upper edge of cross-section:

design value of the vertical single load  $F_{z,Ed} = 400.0$  kN, loading length  $s_s = 300.0$  mm

#### partial safety factors for material

resistance of cross-sections  $\gamma_{M0} = 1.00$

resistance of members in stability failure  $\gamma_{M1} = 1.10$

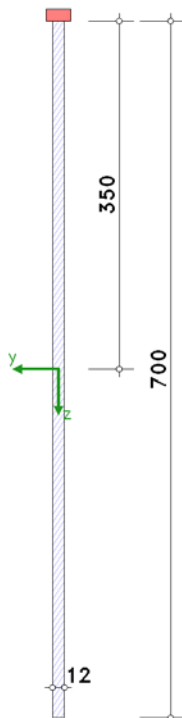


### verifications of buckling resistance

assumption: flange induced web buckling is excluded.

assumption: local buckling of stiffeners is excluded.

assumption: transverse stiffeners serve as rigid support of the plated panel.



**method of reduced stresses**

EC 3-convention, compressive stresses positive

buckling of transverse loading: loading length > distance of two single loads, double load is not investigated !!  
shear distortions are ignored.cross-sectional properties:  $A = 84.00 \text{ cm}^2$ ,  $z_s = 350.0 \text{ mm}$ ,  $I_y = 34300.00 \text{ cm}^4$ ,  $y_s = 0.0 \text{ mm}$ ,  $I_z = 10.08 \text{ cm}^4$   
maximum/minimum stresses:  $\sigma_o = 71.0 \text{ N/mm}^2$ ,  $\sigma_u = -71.0 \text{ N/mm}^2$ ,  $\tau = 29.0 \text{ N/mm}^2$ ,  $\sigma_z = 111.1 \text{ N/mm}^2$ **reduced stresses** $\sigma_{Ed} = 71.0 \text{ N/mm}^2$ ,  $\sigma_{z,Ed} = 111.1 \text{ N/mm}^2$ ,  $\tau_{Ed} = 29.0 \text{ N/mm}^2$ non-dimensional slenderness ratio  $\lambda_p = \lambda_c = \lambda_w = (\alpha_{ult}/\alpha_{cr})^{1/2} = 0.983$ ,  $\alpha_{ult} = 2.143$ ,  $\alpha_{cr} = 2.218$ reduction factor  $\rho = (\lambda_p - 0.055 \cdot (3 + \psi)) / \lambda_p^2 = 0.904 \leq 1$  for  $\lambda_p > 0.5 + (0.085 - 0.055 \cdot \psi)^{1/2} = 0.874$ ,  $\psi = -1.000$ reduction factor  $\rho_z = 1 / (\varphi_p + (\varphi_p^2 - \lambda_p)^{1/2}) = 0.786$  with  $\varphi_p = (1 + \alpha_p \cdot (\lambda_p - \lambda_{p0}) + \lambda_p) / 2 = 1.023$ ,  $\alpha_p = 0.34$ ,  $\lambda_{p0} = 0.8$ critical buckling stress  $\sigma_{cr,p} = \alpha_{cr} \cdot \sigma_{z,Ed} = 246.5 \text{ N/mm}^2$ critical buckling stress  $\sigma_{cr,c} = \pi^2 \cdot E / (12 \cdot (1 - \mu^2)) \cdot (t/b')^2 = 55.8 \text{ N/mm}^2$ ,  $b' = 700.0 \text{ mm}$ reduction factor  $\chi_c = 0.678 \leq 1$  for  $\lambda_c > 0.2$ final reduction factor  $\rho_z = (\rho_z - \chi_c) \cdot \xi \cdot (2 - \xi) + \chi_c = 0.786$  with  $\xi = 1.000$ reduction factor  $\chi_w = 0.83 / \lambda_w = 0.844$  for  $0.83 / \eta = 0.692 \leq \lambda_w < 1.08$ ultimate buckling stresses  $\sigma_{Rd} = \rho \cdot f_y / \gamma_{M1} = 193.0 \text{ N/mm}^2$ ,  $\sigma_{z,Rd} = \rho_z \cdot f_y / \gamma_{M1} = 167.8 \text{ N/mm}^2$ , $\tau_{Rd} = \chi_w \cdot f_y / (3^{1/2} \cdot \gamma_{M1}) = 104.2 \text{ N/mm}^2$ verification:  $((\sigma_{Ed} / \sigma_{Rd})^2 + (\sigma_{z,Ed} / \sigma_{z,Rd})^2 - \rho \cdot \rho_z \cdot (\sigma_{Ed} / \sigma_{Rd}) \cdot (\sigma_{z,Ed} / \sigma_{z,Rd}) + (\tau_{Ed} / \tau_{Rd})^2)^{1/2} = 0.692 < 1$  **ok.**

transverse stiffeners:

rigid support of buckling field:

assumption: transverse stiffeners without axial force.

 $I_{st} = 140.66 \text{ cm}^4 > \sigma_m / E \cdot (b/\pi)^4 \cdot (1 + w_0 \cdot 300 \cdot u/b) = 0.17 \text{ cm}^4$  **ok.**

torsional buckling:

 $I_{T,st} / I_{p,st} = 0.47\% < 5.3 \cdot f_y / E = 0.59\%$  **ok.**

minimum moment of inertia to ensure a rigid support:

 $I_{st} = 707.95 \text{ cm}^4 > 0.75 \cdot h_w \cdot t^3 = 90.72 \text{ cm}^4$  **ok.** for  $a/h_w = 2.86 \geq 2^{1/2}$ total utilization:  $U = 0.692 < 1$  **ok.****Final Result**maximum utilization: max  $U = 0.692 < 1$  **ok.**assumptions: succeeded **ok.****verifications succeeded****Regulations**

DIN EN 1990, Eurocode 0: Grundlagen der Tragwerksplanung;

Deutsche Fassung EN 1990:2002 + A1:2005 + A1:2005/AC:2010, Ausgabe Dezember 2010

DIN EN 1990/NA, Nationaler Anhang zur DIN EN 1990, Ausgabe Dezember 2010

DIN EN 1993-1-1, Eurocode 3: Bemessung und Konstruktion von Stahlbauten -

Teil 1-1: Allgemeine Bemessungsregeln und Regeln für den Hochbau;

Deutsche Fassung EN 1993-1-1:2005 + AC:2009, Ausgabe Dezember 2010

DIN EN 1993-1-1/NA, Nationaler Anhang zur DIN EN 1993-1-1, Ausgabe Dezember 2010

DIN EN 1993-1-5, Eurocode 3: Bemessung und Konstruktion von Stahlbauten -

Teil 1-5: Plattenförmige Bauteile;

Deutsche Fassung EN 1993-1-5:2006 + AC:2009, Ausgabe Dezember 2010

DIN EN 1993-1-5/NA, Nationaler Anhang zur DIN EN 1993-1-5, Ausgabe Dezember 2010