

detailed problems acc. to Eurocode 3

EC 3-1-5 (12.10), NA: EC-standard parameters

steel grade

steel grade S 355

cross-section

beam: parameter (I-section):

$h = 1320.0 \text{ mm}$, $t_w = 20.0 \text{ mm}$, $b_f = 600.0 \text{ mm}$, $t_f = 60.0 \text{ mm}$

longitudinal stiffeners: number $n_{st} = 1$

section parameters (flat steel):

$h = 140.0 \text{ mm}$, $t = 40.0 \text{ mm}$

distance of stiffener to the top edge of beam $d_{st,0} = 940.0 \text{ mm}$

parameters

length of buckling field $a = 240.0 \text{ cm}$

method of effective cross-sectional area

verification in beam field

calculation of buckling factors acc. to EC 3-1-5

effective cross-section values from resulting distribution of longitudinal stresses

loading

internal forces and moments referring to the stiffened cross-section:

transverse loading on lower edge of cross-section:

design value of the vertical single load $F_{z,Ed} = 4000.0 \text{ kN}$, loading length $s_s = 600.0 \text{ mm}$

partial safety factors for material

resistance of cross-sections $\gamma_{M0} = 1.00$

resistance of members in stability failure $\gamma_{M1} = 1.00$

verifications of buckling resistance

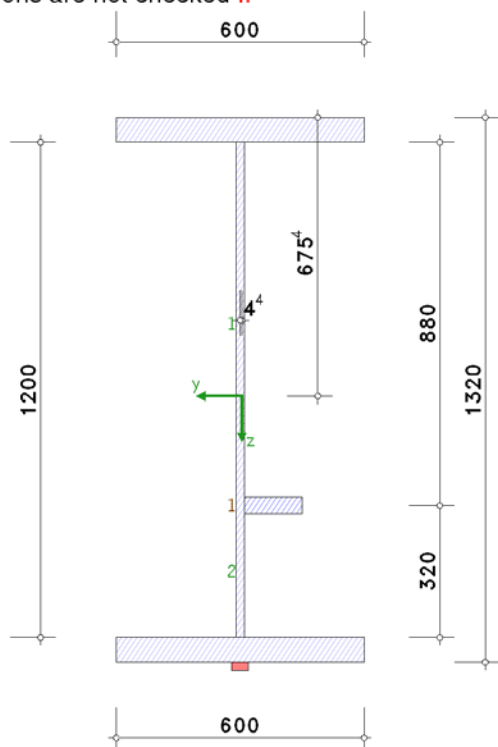
assumption: flange induced web buckling is excluded.

assumption: local buckling of stiffeners is excluded.

assumption: rotational ability of stiffeners for stress redistribution is sufficient.

assumption: plate area is supported rigidly.

the assumptions are not checked !!



Lk 1:

method of effective cross-sectional area

EC 3-convention, compressive stresses positive

buckling of transverse loading: loading length > distance of two single loads, double load is not investigated !!

shear distortions are ignored.

cross-sectional properties: $A = 1016.00 \text{ cm}^2$, $z_s = 675.4 \text{ mm}$, $I_y = 3189398.76 \text{ cm}^4$, $y_s = 4.4 \text{ mm}$, $I_z = 220381.12 \text{ cm}^4$

maximum/minimum stresses: $\sigma_z = 333.3 \text{ N/mm}^2$

buckling of transverse loading

reduction factor $\chi_F = 0.535$

resistance of buckling $F_{z,Rd} = f_y \cdot L_{eff} \cdot t_w / \gamma_{M1} = 5544.35 \text{ kN}$, $L_{eff} = \chi_F \cdot l_y = 780.9 \text{ mm}$, $l_y = 1459.7 \text{ mm}$

verification: $F_{z,Ed} / F_{z,Rd} = 0.721 < 1$ **ok.**

total utilization: $U = 0.721 < 1$ **ok.**

Final Result

maximum utilization: $\max U = 0.721 < 1$ **ok.**

verifications succeeded

Regulations

DIN EN 1990, Eurocode 0: Grundlagen der Tragwerksplanung;

Deutsche Fassung EN 1990:2002 + A1:2005 + A1:2005/AC:2010, Ausgabe Dezember 2010

DIN EN 1993-1-1, Eurocode 3: Bemessung und Konstruktion von Stahlbauten -

Teil 1-1: Allgemeine Bemessungsregeln und Regeln für den Hochbau;

Deutsche Fassung EN 1993-1-1:2005 + AC:2009, Ausgabe Dezember 2010

DIN EN 1993-1-5, Eurocode 3: Bemessung und Konstruktion von Stahlbauten -

Teil 1-5: Plattenförmige Bauteile;

Deutsche Fassung EN 1993-1-5:2006 + AC:2009, Ausgabe Dezember 2010