

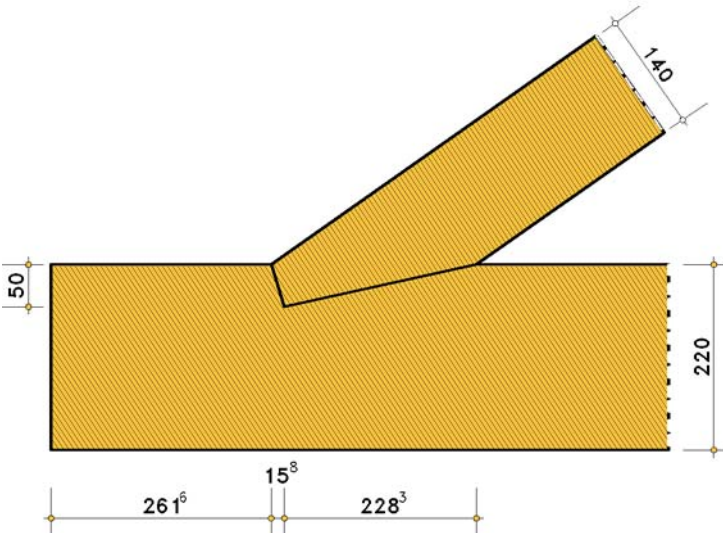
### 1. Input parameters

#### 1.1. frontal offset acc. to DIN EN 1995-1-1/NA:2013-08, NCI NA.12.1

#### 1.2. material and dimensions

both beams from solid coniferous timber, C24 (S10) ,  $\rho_k = 350 \text{ kg/m}^3$ , NKL 1  
 $f_{m,k} = 24.00 \text{ N/mm}^2$ ,  $f_{t,k} = 14.00 \text{ N/mm}^2$ ,  $f_{c,k} = 21.00 \text{ N/mm}^2$ ,  $f_{v,k} = 4.00 \text{ N/mm}^2$ ,  $f_{c90,k} = 2.50 \text{ N/mm}^2$   
 sole plate 120/220 mm, strut 120/140 mm,  $\gamma = 35.0^\circ$   
 anchoring by bolt  $\varnothing 12 \text{ mm}$

elevation scale 1:90, unit of length [mm]



#### 1.3. internal forces and moments

Nr.	name	N <sub>d</sub> kN	KLED	k <sub>mod</sub>	$\gamma$
1	FD	50.00	med.-term	0.800	1.30

### 2. results

#### 2.1. compression in contact surfaces acc. to DIN EN 1995-1-1/NA, NCI NA.12.1

$k_{cr} = 0.500$ ,  $\alpha = \gamma/2 = 17.5^\circ$ ,  $\min l_v = 277 \text{ mm}$

Nr	$f_{v,d}$ N/mm <sup>2</sup>	$f_{c0,d}$ N/mm <sup>2</sup>	$f_{c90,d}$ N/mm <sup>2</sup>	$f_{c\alpha,d}$ N/mm <sup>2</sup>	S <sub>1R,d</sub> kN	l <sub>v</sub> mm	u <sub>lv</sub>	u <sub>SE,d1</sub>	u
1	2.46	12.92	1.54	10.42	68.73	277	0.693	0.727	0.727

$u_{max} = 0.727 \leq 1 \Rightarrow \text{ok.}$

#### 2.2. sole plate bending and normal force

$b_n = 107 \text{ mm}$ ,  $h_n = 170 \text{ mm} \Rightarrow A_n = 18190 \text{ mm}^2$ ,  $W_n = 515383 \text{ mm}^3$ ,  $e_z = 25 \text{ mm}$

Nr	left edge								right edge					
	$f_{m,d}$ N/mm <sup>2</sup>	$f_{t,d}$ N/mm <sup>2</sup>	$f_{c,d}$ N/mm <sup>2</sup>	N <sub>d</sub> kN	$\sigma_{Nd}$ N/mm <sup>2</sup>	M <sub>d</sub> kNm	$\sigma_{m,d}$ N/mm <sup>2</sup>	$u_\sigma$	N <sub>d</sub> kN	$\sigma_{Nd}$ N/mm <sup>2</sup>	M <sub>d</sub> kNm	$\sigma_{m,d}$ N/mm <sup>2</sup>	$u_\sigma$	u
1	14.77	8.62	12.92	-40.96	-2.252	1.024	1.987	0.165	0.000	0.000	0.000	0.000	0.000	0.165

$u_{max} = 0.165 \leq 1 \Rightarrow \text{ok.}$

#### 2.3. sole plate shear force

$b_n = 107 \text{ mm}$ ,  $h_n = 170 \text{ mm} \Rightarrow A_n = 18190 \text{ mm}^2$

Nr	$f_{v,d}$ N/mm <sup>2</sup>	left edge			right edge			u
		V <sub>d</sub> kN	$\tau_d$ N/mm <sup>2</sup>	$u_\tau$	V <sub>d</sub> kN	$\tau_d$ N/mm <sup>2</sup>	$u_\tau$	
1	2.46	28.679	2.365	0.961	0.000	0.000	0.000	0.961

$u_{max} = 0.961 \leq 1 \Rightarrow \text{ok.}$

## 2.4. strut stability check

$l_{\text{eff}} = 120 \text{ mm}$ ,  $E_{0,05} = 7333 \text{ N/mm}^2$ ,  $G_{0,05} = 460 \text{ N/mm}^2$ ,  $A = 16800 \text{ mm}^2$ ,  $W_y = 392000 \text{ mm}^3$

$I_t = 39034481 \text{ mm}^4$ ,  $\beta_c = 0.200$ ,  $i_y = 40 \text{ mm}$ ,  $i_z = 35 \text{ mm}$ ,  $k_{c,y} = 1.000$ ,  $k_{c,z} = 1.000$ ,  $\sigma_{m,\text{krit}} = 3441 \text{ mm}^3$

$\lambda_y = 2.969$ ,  $\lambda_z = 3.464$ ,  $\lambda_{\text{rel},y} = 0.051$ ,  $\lambda_{\text{rel},z} = 0.059$ ,  $\lambda_{\text{rel},m} = 0.084$ ,  $k_{\text{krit}} = 1.000$

offset at both ends of the strut on the opposite side  $\Rightarrow e_z = 45 \text{ mm}$  at the ends of beam

Nr	$f_{m,d}$ N/mm <sup>2</sup>	$f_{t,d}$ N/mm <sup>2</sup>	$f_{c,d}$ N/mm <sup>2</sup>	$F_{c,d}$ kN	$M_{y,d}$ kNm	$\sigma_{c,d}$ N/mm <sup>2</sup>	$\sigma_{m,d}$ N/mm <sup>2</sup>	$u_{\sigma}$ -	$u_{\sigma y r}$ -	$u_{\sigma z r}$ -	$u$ -
1	14.77	8.62	12.92	50.000	2.250	2.976	5.740	<b>0.442</b>	<b>0.230</b>	<b>0.230</b>	<b>0.442</b>

$u_{\text{max}} = 0.442 \leq 1 \Rightarrow \text{ok.}$

## 3. Summary

total utilization all verifications  $u_{\text{max,Ges}} = 0.961 \leq 1 \Rightarrow \text{ok.}$