

1. Input parameters

1.1. frontal offset acc. to DIN EN 1995-1-1/NA:2013-08, NCI NA.12.1

1.2. material and dimensions

sole plate of glue laminated timber EC, GL28h , $\rho_k = 425 \text{ kg/m}^3$, NKL 1

$f_{m,k} = 28.00 \text{ N/mm}^2$, $f_{t,k} = 22.30 \text{ N/mm}^2$, $f_{c,k} = 28.00 \text{ N/mm}^2$, $f_{v,k} = 3.50 \text{ N/mm}^2$, $f_{c90,k} = 2.50 \text{ N/mm}^2$

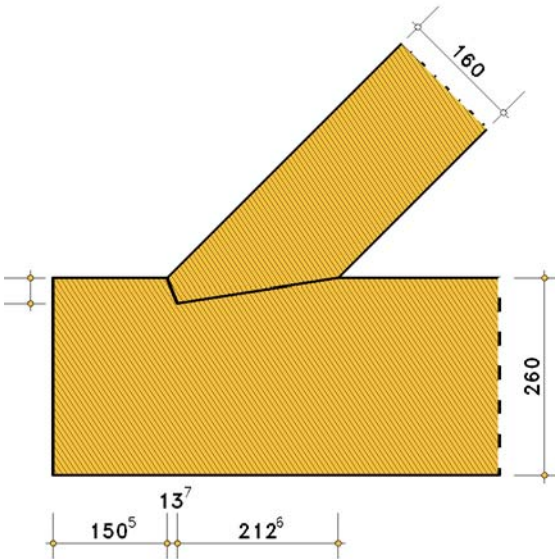
strut from solid coniferous timber, C24 (S10) , $\rho_k = 350 \text{ kg/m}^3$, NKL 1

$f_{m,k} = 24.00 \text{ N/mm}^2$, $f_{t,k} = 14.00 \text{ N/mm}^2$, $f_{c,k} = 21.00 \text{ N/mm}^2$, $f_{v,k} = 4.00 \text{ N/mm}^2$, $f_{c90,k} = 2.50 \text{ N/mm}^2$

sole plate 160/260 mm, strut 160/160 mm, $\gamma = 45.0^\circ$

anchoring by bolt $\varnothing 12 \text{ mm}$

elevation scale 1:100, unit of length [mm]



1.3. internal forces and moments

Nr.	name	N _d kN	KLED	k _{mod} -	γ -
1	A	64.30	sh.-term	0.900	1.30

2. results

2.1. compression in contact surfaces acc. to DIN EN 1995-1-1/NA, NCI NA.12.1

$k_{cr} = 0.714$, $\alpha = \gamma/2 = 22.5^\circ$, $\min l_v = 164 \text{ mm}$

Nr	$f_{v,d}$ N/mm ²	$f_{c0,d}$ N/mm ²	$f_{c90,d}$ N/mm ²	$f_{c\alpha,d}$ N/mm ²	S _{1R,d} kN	l _v mm	u _{1v} -	USE _{d1} -	u -
1	2.42	19.38	1.73	10.36	64.10	164	0.622	1.003	1.003

$u_{max} = 1.003 > 1 \Rightarrow$ **not ok. !!**

2.2. sole plate bending and normal force

$b_n = 147 \text{ mm}$, $h_n = 227 \text{ mm} \Rightarrow A_n = 33369 \text{ mm}^2$, $W_n = 1262461 \text{ mm}^3$, $e_z = 17 \text{ mm}$

Nr	$f_{m,d}$ N/mm ²	$f_{t,d}$ N/mm ²	$f_{c,d}$ N/mm ²	left edge					right edge					u -	
				N _d kN	σ_{Nd} N/mm ²	M _d kNm	$\sigma_{m,d}$ N/mm ²	u_σ -	N _d kN	σ_{Nd} N/mm ²	M _d kNm	$\sigma_{m,d}$ N/mm ²	u_σ -		
1	19.38	15.44	19.38	-45.47	-1.363	0.750	0.594	0.036	0.000	0.000	0.000	0.000	0.000	0.000	0.036

$u_{max} = 0.036 \leq 1 \Rightarrow$ **ok.**

2.3. sole plate shear force

$b_n = 147 \text{ mm}$, $h_n = 227 \text{ mm} \Rightarrow A_n = 33369 \text{ mm}^2$

Nr	$f_{v,d}$ N/mm ²	left edge			right edge			u -
		V _d kN	τ_d N/mm ²	u_τ -	V _d kN	τ_d N/mm ²	u_τ -	
1	2.42	45.467	2.044	0.843	0.000	0.000	0.000	0.843

$u_{max} = 0.843 \leq 1 \Rightarrow$ **ok.**

2.4. strut stability check

$l_{\text{eff}} = 2828 \text{ mm}$, $E_{0,05} = 7333 \text{ N/mm}^2$, $G_{0,05} = 460 \text{ N/mm}^2$, $A = 25600 \text{ mm}^2$, $W_y = 682667 \text{ mm}^3$
 $I_t = 92187307 \text{ mm}^4$, $\beta_c = 0.200$, $i_y = 46 \text{ mm}$, $i_z = 46 \text{ mm}$, $k_{c,y} = 0.657$, $k_{c,z} = 0.657$, $\sigma_{m,\text{krit}} = 212 \text{ mm}^3$
 $\lambda_y = 61.228$, $\lambda_z = 61.228$, $\lambda_{\text{rel},y} = 1.043$, $\lambda_{\text{rel},z} = 1.043$, $\lambda_{\text{rel},m} = 0.336$, $k_{\text{krit}} = 1.000$
offset at both ends of the strut auf der gleichen Seite $\Rightarrow e_z = 64 \text{ mm}$ über die gesamte Stablänge

Nr	$f_{m,d}$ N/mm ²	$f_{t,d}$ N/mm ²	$f_{c,d}$ N/mm ²	$F_{c,d}$ kN	$M_{y,d}$ kNm	$\sigma_{c,d}$ N/mm ²	$\sigma_{m,d}$ N/mm ²	u_{σ} -	$u_{\sigma y r}$ -	$u_{\sigma z r}$ -	u -
1	16.62	9.69	14.54	64.300	4.083	2.512	5.981	0.000	0.623	0.392	0.623

$u_{\text{max}} = 0.623 \leq 1 \Rightarrow \text{ok.}$

3. Summary

total utilization all verifications $u_{\text{max,Ges}} = 1.003 \leq 1 \Rightarrow \text{ok.}$