

# POS. 12: TYPED IK-CONNECTION

## standard IK-connection

hinged IK-connection acc. to EC 3-1-8 (12.10), NA: Deutschland

the connection type, dimensions of beam, bolts, end-plate resp. angle and material are taken of the following literature:

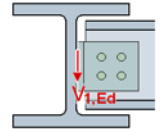
'Typisierte Anschlüsse im Stahlhochbau nach DIN EN 1993-1-8, Stahlbau Verlags- und Service GmbH, Ausgabe 2013' the current number and associated parameters are recorded. verification method is 'elastic-elastic'.

code IK, steel grade S 235

181: beam section IPE330, connection type 2  
notch: a = 80 mm, e = 35 mm, h<sub>a</sub> = 260 mm, d = 17 mm

V<sub>j,Ed</sub>: internal forces and moments at hinge

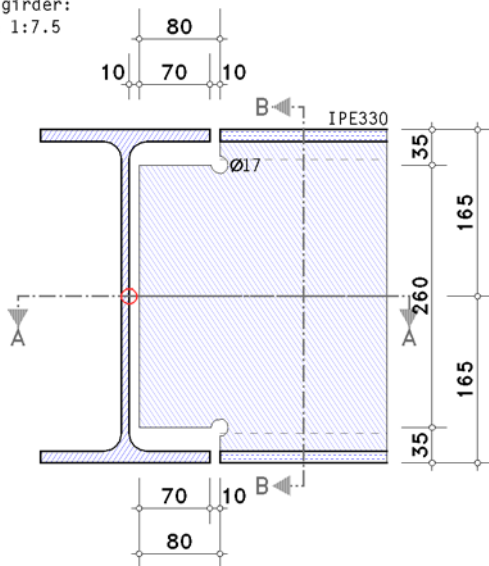
Lk	V <sub>j,Ed</sub> kN		Lk	V <sub>j,Ed</sub> kN		Lk	V <sub>j,Ed</sub> kN	
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1	-68.00	min N	5	-4.22	max V <sub>ζ</sub>	9	-7.76	max V <sub>η</sub>
2	-23.21	max N	6	-95.92	min N	10	-116.87	min V <sub>ζ</sub>
3	-22.74	min V <sub>η</sub>	7	-33.27	max N	11	-111.22	min T
4	-79.40	min V <sub>ζ</sub>	8	-87.48	min V <sub>η</sub>	12	-42.68	max T



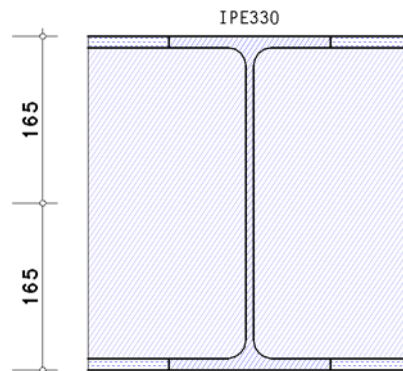
## Simple Joint of Beams

EC 3-1-8 (12.10), NA: Deutschland

main girder:  
scale 1:7.5

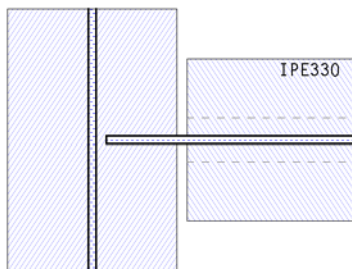


section B-B:



○ hinge

section A-A:



steel grade S 235  
bolts M5-4.6

### partial safety factors for material

resistance of cross sections  $\gamma_{M0} = 1.00$

resistance of bolts, welds, plates in bearing  $\gamma_{M2} = 1.25$

resistance with tension loads  $\gamma_{Mu} = 1.10$

## Final Result

maximum utilization [Lk 10]: design resistance max  $U = 0.709 < 1$  **ok.**

verification succeeded

## Decisive load case combination

### note

design resistance of the main girder is not verified.

### Lk 10: min $V\zeta$

## design values

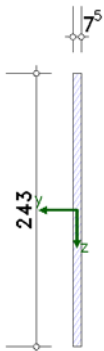
transformation of member forces to the reference point (intersection point of beam axis')

$M_{1,Ed} = V_{j1,Ed} \cdot e_1 = 0.44 \text{ kNm}$ ,  $e_1 = -3.8 \text{ mm}$

$V_{1,Ed} = V_{j1,Ed} = -116.87 \text{ kN}$

## cross-sectional check of supported beam

verification at  $\Delta x = 93.8 \text{ mm}$  (ref. to mg-axis) with 2 notchs



plastic cross-sectional check is not possible  $\Rightarrow$  elastic cross-sectional check

elastic cross-sectional check for  $M_y = -10.52 \text{ kNm}$ ,  $V_z = -116.87 \text{ kN}$

max  $\sigma_v$  bei  $y = -1.9 \text{ mm}$ ,  $z = -0.0 \text{ mm}$ :  $\sigma_x = 0.00 \text{ kN/cm}^2$ ,  $\tau = 9.62 \text{ kN/cm}^2$ ,  $\sigma_v = 16.66 \text{ kN/cm}^2$

verification:  $\sigma_v = 16.66 \text{ kN/cm} < \text{zul } \sigma_v = 23.50 \text{ kN/cm} \Rightarrow U = 0.709 < 1$  **ok.**

utilizations: design resistance  $U_\sigma = 0.709 < 1$  **ok.**

### verification result

maximum utilization: max  $U = 0.709 < 1$  **ok.**

## Regulations

DIN EN 1990, Eurocode 0: Grundlagen der Tragwerksplanung;

Deutsche Fassung EN 1990:2002 + A1:2005 + A1:2005/AC:2010, Ausgabe Dezember 2010

DIN EN 1990/NA, Nationaler Anhang zur DIN EN 1990, Ausgabe Dezember 2010

DIN EN 1993-1-1, Eurocode 3: Bemessung und Konstruktion von Stahlbauten -

Teil 1-1: Allgemeine Bemessungsregeln und Regeln für den Hochbau;

Deutsche Fassung EN 1993-1-1:2005 + AC:2009, Ausgabe Dezember 2010

DIN EN 1993-1-1/NA, Nationaler Anhang zur DIN EN 1993-1-1, Ausgabe Dezember 2010

DIN EN 1993-1-8, Eurocode 3: Bemessung und Konstruktion von Stahlbauten -

Teil 1-8: Bemessung von Anschlüssen;

Deutsche Fassung EN 1993-1-8:2005 + AC:2009, Ausgabe Dezember 2010  
DIN EN 1993-1-8/NA, Nationaler Anhang zur DIN EN 1993-1-8, Ausgabe Dezember 2010

ECCS Document No. 126: European Recommendations for the Design of Simple Joints in Steel Structures.  
ECCS TC10 - Structural Connections, 2009. J.P. Jaspart, J.F. Démonceau, S. Renkin, M.L. Guillaume

Klaus Weynand, Ralf Oerder: Typisierte Anschlüsse im Stahlhochbau nach DIN EN 1993-1-8,  
IS - Gelenkige Stirnplattenanschlüsse, IW - Gelenkige Winkelanschlüsse  
IG - Gelenkige Winkelanschlüsse mit großem Spalt, IK - Ausklinkungen,  
Stahlbau Verlags- und Service GmbH, Ausgabe 2013