

POS. 6: 4 BOLTS

standardized IM-joint

moment resistant joints IM acc. to EC 3-1-8 (12.10), NA: Deutschland

dimensions of beam, bolts, end-plate and welds, material and arrangement of bolts are taken of the following literature:

'Typisierte Anschlüsse im Stahlhochbau nach DIN EN 1993-1-8, Ergänzungsband 2018, Stahlbau Verlags- und Service GmbH, Ausgabe 2018'

the current number and associated parameters are recorded.

MN-interaction follows Cerfontaine (in Jaspart/Weynand: Design of Joints in Steel Structures).

connections with 4 bolts per row are verified with AIF-Forschungsbericht Nr. 15059.

maximum resistance of normal forces are calculated without components of compression/shear and linearization.

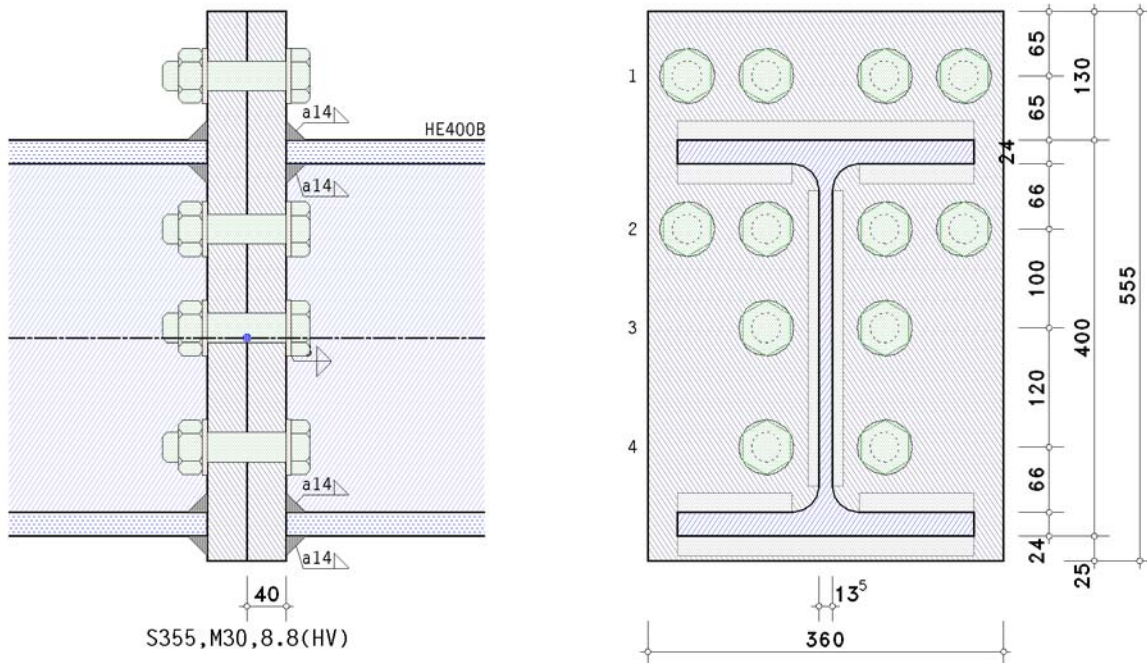
beam splice, steel grade S355, bolt class of bolts 8.8

41291: beam section HEB400, bolt size M30, connection with 4 and 2 bolts per row

end-plate: $t_p = 40$ mm, $b_p = 360$ mm, $h_p = 555$ mm, $e_1 = 65$ mm, $p_{1,1} = 155$ mm, $p_{1,2} = 100$ mm
 $p_{1,3} = 120$ mm, $u_1 = 130$ mm, $w = 120$ mm, $p_2 = 80$ mm

fillet welds: $a_f = 14$ mm, $a_w = 8$ mm

Rigid beam splice



Lk 1: internal moment (top tension) + shear force

resistance of cross-section

$M_{pl,N,Q} = 1130.44$ kNm

connection capacity

moment resistance

resistance per bolt-row

row 1 ($h_r = 453.0$ mm): $F_{tr,Rd} = 1180.2$ kN

row 2 ($h_r = 298.0$ mm): $F_{tr,Rd} = 1127.0$ kN

row 3 ($h_r = 198.0$ mm): $F_{tr,Rd} = 646.3$ kN

row 4 ($h_r = 78.0$ mm): $F_{tr,Rd} = 97.9$ kN

resistance of flanges

$F_{c,Rd} = 3051.5$ kN

moment resistance

$M_{j,Rd} = \sum(F_{tr,Rd} \cdot h_r) = 1006.1$ kNm

$N_{j,t,Rd} = \sum F_{tr,Rd}^* = 3599.8$ kN

$N_{j,c,Rd} = F_{c,Rd} = 3051.5$ kN

shear/bearing resistance

resistance per bolt-row

row 1: $F_{vr,Rd} = 155.1 \text{ kN}$
row 2: $F_{vr,Rd} = 155.1 \text{ kN}$
row 3: $F_{vr,Rd} = 155.1 \text{ kN}$
row 4: $F_{vr,Rd} = 155.1 \text{ kN}$
shear/bearing resistance
 $V_{j,Rd} = \Sigma F_{vr,Rd} = 620.4 \text{ kN}$

shear resistance

shear resistance of end plate: $V_{ep,Rd} = 1199.00 \text{ kN}$
plastic shear resistance $V_{pl,Rd} = 717.1 \text{ kN}$ (requirement, s. 'Typisierte Anschlüsse')

total

$M_{j,Rd} = 1006.1 \text{ kNm}$ $N_{j,t,Rd} = 3599.8 \text{ kN}$ $N_{j,c,Rd} = 3051.5 \text{ kN}$ $V_{j,Rd} = 620.4 \text{ kN}$ $V_{pl,Rd} = 717.1 \text{ kN}$ $V_{ep,Rd} = 1199.0 \text{ kN}$

rotational stiffness

initial rotational stiffness: $S_{j,ini} = (E \cdot z^2) / \Sigma(1/k_i) = 841850.5 \text{ kNm/rad}$, $\Sigma(1/k_i) = 0.030 \text{ mm}^{-1}$
 $lM_{j,Ed} = 1.00 \text{ kNm} \leq 2/3 M_{j,Rd} = 670.7 \text{ kNm} \Rightarrow \mu = 1$
rotational stiffness: $S_{j,Rd} = S_{j,ini} / \mu = 841850.5 \text{ kNm/rad}$
rotation: $\varphi_{j,Ed} = M_{j,Ed} / S_{j,Rd} = 0.000^\circ$

Lk 2: internal moment (bottom tension) + shear force

resistance of cross-section

$M_{pl,N,Q} = 1130.44 \text{ kNm}$

connection capacity

moment resistance

resistance per bolt-row
row 1 ($h_r = 298.0 \text{ mm}$): $F_{tr,Rd} = 646.3 \text{ kN}$
row 2 ($h_r = 178.0 \text{ mm}$): $F_{tr,Rd} = 386.0 \text{ kN}$
row 3 ($h_r = 78.0 \text{ mm}$): $F_{tr,Rd} = 169.2 \text{ kN}$

resistance of flanges

$F_{c,Rd} = 3051.5 \text{ kN}$

moment resistance

$M_{j,Rd} = \Sigma(F_{tr,Rd} \cdot h_r) = 274.5 \text{ kNm}$

$N_{j,t,Rd} = \Sigma F_{tr,Rd}^* = 1938.8 \text{ kN}$

$N_{j,c,Rd} = F_{c,Rd} = 3051.5 \text{ kN}$

shear/bearing resistance

resistance per bolt-row
row 1: $F_{vr,Rd} = 155.1 \text{ kN}$
row 2: $F_{vr,Rd} = 155.1 \text{ kN}$
row 3: $F_{vr,Rd} = 155.1 \text{ kN}$
row 4: $F_{vr,Rd} = 155.1 \text{ kN}$

shear/bearing resistance

$V_{j,Rd} = \Sigma F_{vr,Rd} = 620.4 \text{ kN}$

shear resistance

shear resistance of end plate: $V_{ep,Rd} = 1199.00 \text{ kN}$
plastic shear resistance $V_{pl,Rd} = 717.1 \text{ kN}$ (requirement, s. 'Typisierte Anschlüsse')

total

$M_{j,Rd} = 274.5 \text{ kNm}$ $N_{j,t,Rd} = 1938.8 \text{ kN}$ $N_{j,c,Rd} = 3051.5 \text{ kN}$ $V_{j,Rd} = 620.4 \text{ kN}$ $V_{pl,Rd} = 717.1 \text{ kN}$ $V_{ep,Rd} = 1199.0 \text{ kN}$

rotational stiffness

initial rotational stiffness: $S_{j,ini} = (E \cdot z^2) / \Sigma(1/k_i) = 190828.4 \text{ kNm/rad}$, $\Sigma(1/k_i) = 0.059 \text{ mm}^{-1}$
 $lM_{j,Ed} = 1.00 \text{ kNm} \leq 2/3 M_{j,Rd} = 183.0 \text{ kNm} \Rightarrow \mu = 1$
rotational stiffness: $S_{j,Rd} = S_{j,ini} / \mu = 190828.4 \text{ kNm/rad}$
rotation: $\varphi_{j,Ed} = M_{j,Ed} / S_{j,Rd} = 0.000^\circ$

resistance of cross-section

$$M_{pl,N,Q} = 1130.44 \text{ kNm}$$

connection capacity**bending/tension resistance**

resistance per bolt-row

row 1 ($h_r = 453.0 \text{ mm}$): $F_{tr,Rd} = 0.0 \text{ kN}$

row 2 ($h_r = 298.0 \text{ mm}$): $F_{tr,Rd} = 646.3 \text{ kN}$

row 3 ($h_r = 198.0 \text{ mm}$): $F_{tr,Rd} = 429.4 \text{ kN}$

row 4 ($h_r = 78.0 \text{ mm}$): $F_{tr,Rd} = 169.2 \text{ kN}$

moment resistance

$$M_{j,Rd} = \Sigma(F_{tr,Rd} \cdot h_r) = 290.8 \text{ kNm}$$

$$N_{j,t,Rd} = \Sigma F_{tr,Rd}^* = 1938.8 \text{ kN}$$

shear/bearing resistance

resistance per bolt-row

row 1: $F_{vr,Rd} = 155.1 \text{ kN}$

row 2: $F_{vr,Rd} = 155.1 \text{ kN}$

row 3: $F_{vr,Rd} = 155.1 \text{ kN}$

row 4: $F_{vr,Rd} = 155.1 \text{ kN}$

shear/bearing resistance

$$V_{j,Rd} = \Sigma F_{vr,Rd} = 620.4 \text{ kN}$$

shear resistanceshear resistance of end plate: $V_{ep,Rd} = 1199.00 \text{ kN}$ plastic shear resistance $V_{pl,Rd} = 717.1 \text{ kN}$ (requirement, s. 'Typisierte Anschlüsse')**total**

$$M_{j,Rd} = 290.8 \text{ kNm} \quad N_{j,t,Rd} = 1938.8 \text{ kN} \quad V_{j,Rd} = 620.4 \text{ kN} \quad V_{pl,Rd} = 717.1 \text{ kN} \quad V_{ep,Rd} = 1199.0 \text{ kN}$$

rotational stiffness

rotational stiffness only für bending connections !!

resistance of cross-section

$$M_{pl,N,Q} = 1130.44 \text{ kNm}$$

connection capacity**bending/tension resistance**

resistance per bolt-row

row 1 ($h_r = 298.0 \text{ mm}$): $F_{tr,Rd} = 646.3 \text{ kN}$

row 2 ($h_r = 178.0 \text{ mm}$): $F_{tr,Rd} = 386.0 \text{ kN}$

row 3 ($h_r = 78.0 \text{ mm}$): $F_{tr,Rd} = 169.2 \text{ kN}$

row 4 ($h_r = -77.0 \text{ mm}$): $F_{tr,Rd} = 0.0 \text{ kN}$

moment resistance

$$M_{j,Rd} = \Sigma(F_{tr,Rd} \cdot h_r) = 274.5 \text{ kNm} \quad \text{for } h_r \geq 0$$

$$N_{j,t,Rd} = \Sigma F_{tr,Rd}^* = 1938.8 \text{ kN}$$

shear/bearing resistance

resistance per bolt-row

row 1: $F_{vr,Rd} = 155.1 \text{ kN}$

row 2: $F_{vr,Rd} = 155.1 \text{ kN}$

row 3: $F_{vr,Rd} = 155.1 \text{ kN}$

row 4: $F_{vr,Rd} = 155.1 \text{ kN}$

shear/bearing resistance

$$V_{j,Rd} = \Sigma F_{vr,Rd} = 620.4 \text{ kN}$$

shear resistanceshear resistance of end plate: $V_{ep,Rd} = 1199.00 \text{ kN}$ plastic shear resistance $V_{pl,Rd} = 717.1 \text{ kN}$ (requirement, s. 'Typisierte Anschlüsse')**total**

$$M_{j,Rd} = 274.5 \text{ kNm} \quad N_{j,t,Rd} = 1938.8 \text{ kN} \quad V_{j,Rd} = 620.4 \text{ kN} \quad V_{pl,Rd} = 717.1 \text{ kN} \quad V_{ep,Rd} = 1199.0 \text{ kN}$$

rotational stiffness

rotational stiffness only für bending connections !!

Lk 5: compression force + internal moment (top tension)

resistance of cross-section

$M_{pl,N,Q} = 1130.44 \text{ kNm}$

connection capacity

bending/comproession resistance

resistance per bolt-row

row 1 ($h_r = 453.0 \text{ mm}$): $F_{tr,Rd} = 646.3 \text{ kN}$

row 2 ($h_r = 298.0 \text{ mm}$): $F_{tr,Rd} = 425.1 \text{ kN}$

row 3 ($h_r = 198.0 \text{ mm}$): $F_{tr,Rd} = 282.5 \text{ kN}$

row 4 ($h_r = 78.0 \text{ mm}$): $F_{tr,Rd} = 111.3 \text{ kN}$

resistance of flanges

$\Sigma F_{c,Rd} = 3051.5 + 3051.5 = 6103.0 \text{ kN}$

moment resistance

$M_{j,Rd} = \Sigma(F_{tr,Rd} \cdot h_r) = 484.1 \text{ kNm}$

$N_{j,c,Rd} = \Sigma F_{c,Rd} = 6103.0 \text{ kN}$

shear/bearing resistance

resistance per bolt-row

row 1: $F_{vr,Rd} = 155.1 \text{ kN}$

row 2: $F_{vr,Rd} = 155.1 \text{ kN}$

row 3: $F_{vr,Rd} = 155.1 \text{ kN}$

row 4: $F_{vr,Rd} = 155.1 \text{ kN}$

shear/bearing resistance

$V_{j,Rd} = \Sigma F_{vr,Rd} = 620.4 \text{ kN}$

shear resistance

shear resistance of end plate: $V_{ep,Rd} = 1199.00 \text{ kN}$

plastic shear resistance $V_{pl,Rd} = 717.1 \text{ kN}$ (requirement, s. 'Typisierte Anschlüsse')

total

$M_{j,Rd} = 484.1 \text{ kNm}$ $N_{j,c,Rd} = 6103.0 \text{ kN}$ $V_{j,Rd} = 620.4 \text{ kN}$ $V_{pl,Rd} = 717.1 \text{ kN}$ $V_{ep,Rd} = 1199.00 \text{ kN}$

rotational stiffness

rotational stiffness only für bending connections !!

Lk 6: compression force + internal moment (bottom tension)

resistance of cross-section

$M_{pl,N,Q} = 1130.44 \text{ kNm}$

connection capacity

bending/comproession resistance

resistance per bolt-row

row 1 ($h_r = 298.0 \text{ mm}$): $F_{tr,Rd} = 646.3 \text{ kN}$

row 2 ($h_r = 178.0 \text{ mm}$): $F_{tr,Rd} = 386.0 \text{ kN}$

row 3 ($h_r = 78.0 \text{ mm}$): $F_{tr,Rd} = 169.2 \text{ kN}$

resistance of flanges

$\Sigma F_{c,Rd} = 3051.5 + 3051.5 = 6103.0 \text{ kN}$

moment resistance

$M_{j,Rd} = \Sigma(F_{tr,Rd} \cdot h_r) = 274.5 \text{ kNm}$

$N_{j,c,Rd} = \Sigma F_{c,Rd} = 6103.0 \text{ kN}$

shear/bearing resistance

resistance per bolt-row

row 1: $F_{vr,Rd} = 155.1 \text{ kN}$

row 2: $F_{vr,Rd} = 155.1 \text{ kN}$

row 3: $F_{vr,Rd} = 155.1 \text{ kN}$

row 4: $F_{vr,Rd} = 155.1 \text{ kN}$

shear/bearing resistance

$V_{j,Rd} = \Sigma F_{vr,Rd} = 620.4 \text{ kN}$

shear resistance

shear resistance of end plate: $V_{ep,Rd} = 1199.00 \text{ kN}$

plastic shear resistance $V_{pl,Rd} = 717.1 \text{ kN}$ (requirement, s. 'Typisierte Anschlüsse')

total

$M_{j,Rd} = 274.5 \text{ kNm}$ $N_{j,c,Rd} = 6103.0 \text{ kN}$ $V_{j,Rd} = 620.4 \text{ kN}$ $V_{pl,Rd} = 717.1 \text{ kN}$ $V_{ep,Rd} = 1199.0 \text{ kN}$

rotational stiffness

rotational stiffness only für bending connections !!

Final Result

initial stiffness:	$S_{j,ini} = 841.9 \text{ MNm/rad}$
moment resistance (M+):	$M_{j1,Rd} = 1006.1 \text{ kNm}$
moment resistance (M-):	$M_{j2,Rd} = 274.5 \text{ kNm}$
tension resistance:	$N_{jt,Rd} = 1938.8 \text{ kNm}$
compression resistance:	$N_{jc,Rd} = 6103.0 \text{ kNm}$
shear force resistance:	$V_{j,Rd} = 620.4 \text{ kNm}$
moment resistance of beam section:	$M_{c,Rd} = 1130.4 \text{ kNm}$